

## **Abstract**

A method is given for determining the bearing capacity of driven piles in aggressive soil conditions. The shear resistance f0 of concrete corrosion products and the resistance of piles Фk in the area of possible corrosion damage to concrete were determined. Taking into account the influencing factor of the corrosion process on the bearing capacity of piles in medium, highly aggressive environments, primary and secondary protection of piles have been proposed.

**Keywords**: Saline soils, soil resistance along the lateral surface of the pile, concrete corrosion, predicted corrosion depth, impregnating compositions, sulfate-resistant рortland cement.

## **Introduction**

Soils and groundwater in many areas of Central Fergana are characterized by the content of watersoluble salts that have an aggressive effect on the foundation material. For the construction of aboveground structures in conditions of salinization of the surface layer of soils, an important task is to impart resistance and protect against corrosion of the structures of the foundations of buildings and structures [1,2,3,5]. Recently, pile foundations have been used for agricultural buildings in aggressive soil conditions [7]. The effectiveness of various methods for increasing the resistance and anti-corrosion protection of reinforced concrete elements in saline soils is largely determined by the reliability of the assessment of the type and degree of aggressiveness of the environment and the forecast of changes in aggressiveness during operation. It has been established that corrosion of the surface of the pile material affects their bearing capacity [8,9,10]. In this case, the influence of the factor depends on the degree of stress-strain state of the near-pile space, the pile material, the aggressiveness and temperature of the environment, as well as the filtration rate of ground or surface water. The work indicates [5] that in the process of interaction between an aggressive environment and cement stone, calcium oxide, which makes up 60...70% of cement stone, is neutralized, as well as oxides (silica, alumina and iron oxide), which provide a significant effect on the shear resistance of corrosion products. Based on this, it should be expected that the sought values of shear resistance of concrete corrosion can be conveniently determined in laboratory conditions by testing corrosion products on a single-plane shear device GGP-30, taking into account the methodology [4]. The studies were conducted in an aggressive environment typical for the soils and groundwater of

Central Fergana. The composition and content of aggressive ions in the solution are as follows:



*SO<sup>11</sup> <sup>4</sup>=3520mgr/l; cl<sup>1</sup>=105mgr/l;HCO<sup>1</sup> <sup>3</sup>=10 mgr/l;Ca<sup>11</sup>=230 mgr/l; Mg<sup>11</sup>=30 mgr/l;Na<sup>1</sup> +K<sup>1</sup>= 3485 mgr/l.*



Below are the results of the tests of corrosion products of the foundation material.

Based on the obtained data, the values of shear resistance  $f_{\theta}$  of concrete corrosion products within the normal pressure  $\sigma = 0$ ÷0.3MPa can be represented in the following form:

1. 
$$
f_0 = 0,05\sigma + 7
$$
 (1)  
From expression (1) it can be seen that with increasing compressive stresses the influence of  
concrete corrosion products on the bearing capacity of the pile decreases. If we take into account  
that the most active corrosion processes occur in the aeration zone (zones of alternating wetting and  
drying), then the influence of the above factor occurs within certain limits along the length of the  
piles. It is obvious that the predicted area of active corrosion along the length of the piles should be  
assigned according to the lithological structure of the soil, the groundwater level (including the  
predicted one) and the aggressiveness of the environment. Based on this, the determination of the  
resistance along the lateral surface of the piles  $f_0$  in the area of active concrete corrosion in  
accordance with the lateral soil pressure at the contact of the piles with the soil at the depth under  
consideration. The pile resistances in the area of possible corrosion damage to concrete  $F_k$ , taking  
into account (1), can be written as:

$$
2. \qquad F_{\kappa} = m_{\kappa} u_{k} \left( 0.05 \sigma + 7 \right) l_{k} \tag{2}
$$

3.  $m_k$  – safety factor equal to 1.2;

4.  $u_k$  – pile perimeter taking into account the predicted concrete corrosion, m;

5. σ – normal pressure acting on the lateral surface of the pile shaft at the considered layer depth;

*6.* lk – thickness of the soil layer in which active concrete corrosion is expected, *m.*

7. Taking into account the influencing factor of the corrosion process on the bearing capacity of piles in medium, highly aggressive environments, methods of their protection are used. Primary protection of concrete and reinforced concrete in aggressive soil environments consists of increasing the ability of concrete, reinforcement and reinforced concrete to resist the effects of a corrosive environment by changing the composition and structure of concrete before or during the manufacture of structures [1,7]. This is achieved by choosing resistant source materials, concrete compositions, additives, types of reinforcement and concrete, methods of manufacturing and calculating structural elements. Secondary protection consists of increasing the corrosion resistance of concrete after its manufacture by partially or completely filling the pore structure with various chemically resistant materials (surface or complete impregnation), installing surface protective coatings or artificially lowering the level of aggressive groundwater. Currently, impregnating compositions based on styrene-indene resin, pyroplast and polyisocyanate K\*, urea-formaldehyde



resin, penetrating hydrocem, bitumen, etc. are widely used for anti-corrosion protection of reinforced concrete piles, foundations and other underground structures in natural highly aggressive soil environments [3]. Depending on the type of groundwater aggressiveness, Portland cement is usually used as a binder for concrete intended for subsequent impregnation. In areas with saline soils and highly mineralized groundwater, sulfate-resistant Portland cements should be used for the manufacture of concrete.

Reinforced concrete piles exposed to dynamic impact loads during driving must be made of dense concrete with a water resistance grade of at least B-6, since a decrease in the initial density of concrete leads to a sharp decrease in the number of impacts before destruction and an increase in micro-violations in the concrete of piles after driving.

When impregnating in areas with a hot climate, it is necessary to provide measures to protect impregnation baths from direct heating by sunlight, since under these conditions the possibility of solvent evaporation increases and, as a consequence, the viscosity of the impregnation composition increases. Before impregnation, the surface of reinforced concrete products is pre-cleaned and dustfree with compressed air.

Before impregnating products in baths, a laboratory test of the impregnation mode is carried out on cube samples with an edge of 10 cm. If after impregnation for 8 hours, when splitting control samples, the depth of the impregnated layer is at least 10 mm, then impregnation of the products is carried out.

The effect of impregnation on the resistance along the lateral surface of piles is determined by the results of pile tests [1]. It is known that when testing reinforced concrete piles with various anticorrosion coatings after driving, the protective properties of the anti-corrosion coatings are reduced. In turn, the protective coatings will have a different effect on the process of driving piles into the ground, and for "hanging" piles on the value of the bearing capacity of the structure as a result of changing the friction of the soil along the lateral surface of the pile. It is known that hanging piles protected with bitumen mastics in clay soils reduce the bearing capacity compared to piles without protection by more than 30%, and when coating piles with urea-formaldehyde resin up to 20% [3]. Based on the data obtained, the parameters taking into account the effect of impregnation of sandy and clayey saline soils are proposed to be taken within 0.8-0.9. Here the lower limit of the parameter corresponds to saline sands.

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